SH 170 From IH 35W to SH 114

Draft Conceptual Alternative Analysis Report



TARRANT & DENTON COUNTIES

March 19, 2008

Prepared by: Carter & Burgess, Inc. For: North Texas Tollway Authority

Table of Contents

- I. Introduction
- II. Conceptual Alternatives Analysis Report Contents
- III. Conceptual Alternatives Analysis Criteria
- IV. Geometric Criteria
- V. Typical Sections
- VI. Design Concept Conference
- VII. Alternatives / Options Analysis

Appendix

- Regional Location Map
- Overall Project Location Map
- Geometric Design Criteria
- Typical Sections
 - o SH 170
 - Cross Streets & SH 114
 - o Comparison of PGL/Superelevation Rotation Location
- Baseline Schematic (TxDOT)
- Options 1-8 Exhibits
- Environmental Constraints Map

SH 170 Conceptual Alternative Analysis Report

I. INTRODUCTION

The North Texas Tollway Authority (NTTA) has contracted with Carter & Burgess, Inc. to perform comprehensive planning services related to project management, data collection, advanced planning, and preliminary design and environmental documentation for the proposed SH 170 toll road project. The project limits are from IH 35W to SH 114 in northern Tarrant and Southern Denton Counties; a project length of approximately 6.3 miles. The county line also represents the boundary of the Fort Worth and Dallas Districts of the Texas Department of Transportation (TxDOT), thus most of the project corridor is located within the Fort Worth District (Tarrant County), while less than a quarter of the project length is located in the Dallas District (Denton County).

A schematic design for SH 170 was prepared by TxDOT in the late 1980's, and this schematic has served as the basis for construction of frontage roads that currently exist along the full length of the project corridor. The original schematic will serve as the baseline for the refinement of the NTTA schematic. Existing adjacent development requires that implementing the original ramping scheme at some locations may require adjustments which are identified as options in this report.

The proposed SH 170 typical section has four twelve-foot travel lanes (two lanes in each direction) with auxiliary lanes between appropriate ramps, 10'-12' shoulders and an NTTA-standard 50' median. A key aspect of the project is to utilize the existing right-of-way (ROW) and control-of-access (COA) as much as is feasible to tie access ramps to and from the existing frontage roads.

II. CONCEPTUAL ALTERNATIVES ANALYSIS REPORT CONTENTS

The Conceptual Alternative Analysis Report contains several items:

- Development of Criteria for Evaluation of Alternatives
- Development of Geometric Criteria and Typical Sections
- Summarize the Results of a Design Concept Conference (DCC)
- Development of Conceptual Options, including:
 - o Major Interchange Configurations
 - Evalution to Changes in Access
 - Preliminary Relative Cost Analyses
 - Tabular Summary of ROW Needs
 - Tabular Comparison of Options

The final version of the report will also take into account the important public involvement input from stakeholder meetings and public meetings. It will also take into account other meetings that provide input to the project development, such as meetings with public officials. Ongoing environmental investigations and traffic projections will provide further documentation for alternatives analysis and any available pertinent information will be included in the final report.

III. CONCEPTUAL ALTERNATIVES ANALYSIS CRITERIA

With the corridor defined by the TxDOT schematic, ROW and Access Control purchased, and existing frontage roads in place, the nature of the alternatives for this project focus primarily on matching the previously planned facility to the ongoing existing development and access. Centerline modifications to SH 170 are not proposed, nor are re-alignment of any of the existing frontage roads.

Therefore, criteria for the various options are focused on mainlane operations, ramp operations, frontage road operations, control of access impacts, anticipated relative cost impacts and safety impacts. Favorable rankings are indicated by shading in the table of options.

IV. GEOMETRIC CRITERIA

Design Criteria have been developed, tabulated, and referenced using the latest edition of the TxDOT *Roadway Design Manual*. Design criteria developed to date have been approved by the NTTA's Project Management Operations (PMO) group for use on the project.

These criteria tables are included in the Appendix for reference.

V. TYPICAL SECTIONS

Typical Sections have been developed for the corridor, reflecting:

- the existing TxDOT schematic design
- frontage roads already in place and operational
- NTTA standard median width of 50'
- PMO direction on cross-slopes and superelevation rotation point / profile grade location.

The TxDOT schematic calls for 2.6% cross-slopes throughout the project, including mainlanes, ramps, and frontage roads. The PMO directed Carter & Burgess to use a standard cross-slope on the mainlanes and ramps to be constructed at 2.5%. Cross-slopes on any SH 114 mainlanes will be kept at the 2.6% planned by TxDOT.

A Profile Grade Line (PGL) and superelevation rotation point was discussed as being at either the centerline or along each inside lane line of the mainlanes. Each has an advantage: rotating about the centerline allows future widening to the center of the mainlanes to produce a singular plane of paving meeting at a center barrier; rotating about the inside lane line on each side shortens superelevation transition lengths and improves clearance efficiencies under structures. Upon review, the PMO directed use of the inside lane line as the PGL and rotation point.

Inside shoulder widths of 12' have also been discussed; the advantage to these is when any widening to the inside of the SH 170 mainlanes is warranted, the paving joint will occur at a lane line as opposed to at a point 2' off the lane line for a 10' inside shoulder.

The 10' inside shoulder puts the future paving joint essentially at the wheel path. To date, the NTTA has looked favorably on aiming to keep future paving joints at lane lines.

Typical Section exhibits are included in the Appendix for reference.

VI. DESIGN CONCEPT CONFERENCE

To date, this meeting has not been held. Once held, meeting agendas, attendance rosters, and notes will be included in the final version of this report.

VII. ALTERNATIVES / OPTIONS ANALYSIS

The corridor is partially constructed to date, based on the previous TxDOT schematic:

- Frontage roads are in place throughout the entire length of the Project Study Area.
- ROW has been purchased and Access Control areas are defined and have been purchased.
- Major grading has been completed to a practical extent.
- Several crossovers have been constructed at-grade to allow circulation between the frontage roads. These crossovers function as either cross-streets or as temporary connector pavement to be removed in the preferred alternative.

Development of the corridor is primarily residential on the western half of the Study Area (West of Park Vista), with a considerable amount of industrial buildings between Park Vista and US 377 on the eastern half of the Study Area.

Driveways accessing the existing frontage roads have been constructed based on the original TxDOT schematic. Over time, the State's direction has defined longer areas for denial of access to enhance safety and traffic operations. As such, there exist along the SH 170 corridor combinations of TxDOT schematic ramp locations and existing access control zones that create areas where existing driveways placed immediately after control of access zones may need adjustments to either driveway location, control of access, ramp terminal adjustment, or some other adjustment measure.

Given the definition of the existing corridor, a majority of the analysis for the alternatives will not define variations on centerline, but rather ramp locations. In some cases moving ramps slightly may alleviate an access control conflict. In other cases, reversing consecutive ramps to a modified configuration will be analyzed. The goal is to maintain as much of the baseline TxDOT schematic, existing frontage roads, and ROW as possible, while not creating excessive areas of access impacts to existing adjacent development.

Following are options for certain areas where access may present design and operations issues.

A. Options 1A, 1B, 1C, 1D

Option 1A proposes to move the WB 170 ramp to NB 35W (access to frontage road and Cabela Dr) east to avoid an access issue at the Cabela Dr intersection with the WB frontage road. This option does reduce the weave distance from the previous ramp, but level-of-service analysis will determine if this is an issue. An auxiliary lane is planned between the two ramps. Options 1B, 1C, and 1D address situations where driveways to adjacent commercial property have been constructed so close to access control zones that applying current TxDOT ramp design criteria would create issues with keeping those driveways open. Additional ROW and/or access rights in the vicinity of these driveways may need to be acquired. Three such existing driveway locations were identified:

- 1. Exit ramp from WB SH 170 to Westport Parkway,
- 2. Exit ramp from EB SH 170 to US 377, and
- 3. Exit ramp from WB SH 170 to Independence Parkway

B. Option 2

Option 2 includes shifting the entrance ramp from North Beach Street to WB SH 170 approximately 1000 feet to the east to allow increased weaving distance between the consecutive Beach St and IH 35W ramps. To reduce cost and minimize number of structures over the creek, this ramp should be shifted far enough to the east to share the proposed bridge of the westbound mainlanes.

C. Option 3

Alta Vista Road within the SH 170 ROW was originally built as a temporary crossover between the eastbound and westbound frontage roads. With existing residential development in this vicinity on both sides of the SH 170 corridor now in place, roadway users use Alta Vista to access the SH 170 frontage roads in both directions for trips to and from the residential areas.

Often, removing a connection that provides access to a high-volume facility receives negative public response. Therefore, anticipating this view on Alta Vista Road, Option 3 allows the existing access to remain by replacing the existing crossover with a proposed bridge over SH 170. A modification to the SH 170 profile grade from the existing TxDOT schematic design would be necessary. However, depressing the SH 170 mainlanes in this area should reduce noise impacts to the residential areas. Additionally, the revised profile grade for this option is gentler than the baseline schematic; operational improvements may be seen by keeping the profile grades at lower values. This option would require retaining walls between the existing frontage roads and the proposed mainlanes, so costs would be higher than the baseline schematic.

D. Option 4

Option 4 includes shifting the exit ramp from WB SH 170 to Westport Parkway to the east. This would eliminate a control of access conflict with the property just west of Westport Parkway. However, moving the ramp east reduces the weaving distance between this ramp and next ramp, the entrance ramp from Independence Parkway to WB SH 170. LOS analysis would determine if this weaving distance remains acceptable.

E. Option 5

Option 5 modifies the schematic ramp configuration eastbound between Independence Parkway and US 377. In this option, the exit ramp to US 377 would be shifted east and the entrance ramp from Independence Parkway would be shifted west. This modification would eliminate some of the control of access issues related to existing driveways in this area. The trade-off for this option is that access to and from some of the properties from the frontage roads in this vicinity would require passing through signals at Independence or US 377. Victory Lane, an existing roadway, could provide indirect access functionality to allow the commercial properties along the EB frontage road between Indpendence and US 377 the ability to enter SH 170 without passing through the US 377 signal.

F. Option 6

Option 6 includes shifting the WB exit ramp to Independence Parkway to the east. While this option would eliminate the control of access issue in this vicinity, it would also slightly reduce the weaving distance between this ramp and entrance ramp just to the east of it.

G. Option 7

Similar to Option 5 but in the westbound direction, Option 7 modifies the westbound ramp configurations between Independence Parkway and US 377. The WB exit ramp to Independence Parkway would shift west while the WB onramp from US 377 would shift east, reversing the ramp arrangement from the TxDOT schematic. While this modification would eliminate control of access issues in this area, access to the properties in this vicinity would then require passing through the signalized intersection at US 377 or Independence Pkwy. Additionally, there is no roadway behind the commercial properties to circle back once exiting SH 170 for Independence Pkwy.

H. Option 8

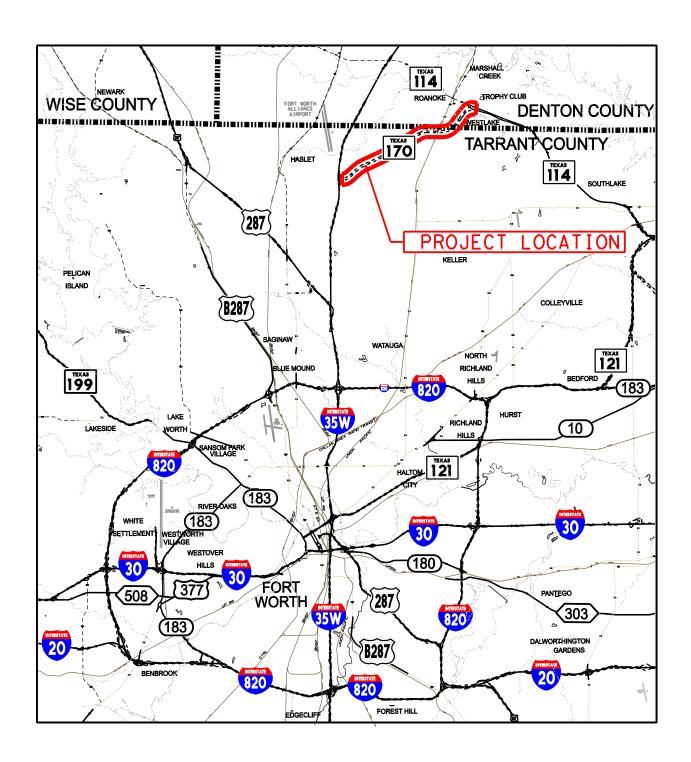
Option 8 consists of adding U-turn bridges in both directions at US 377 intersection. This change would improve the safety and operation of the intersection. However, cost estimates would reflect the two additional bridges constructed over SH 170 for this option. Given the truck traffic that is undoubtedly heavier in this area of the project due to the industrial land use, providing these U-turns may help keep the less agile trucks out of the signalized intersections.

SH 170 Draft Conceptual Alternatives Analysis

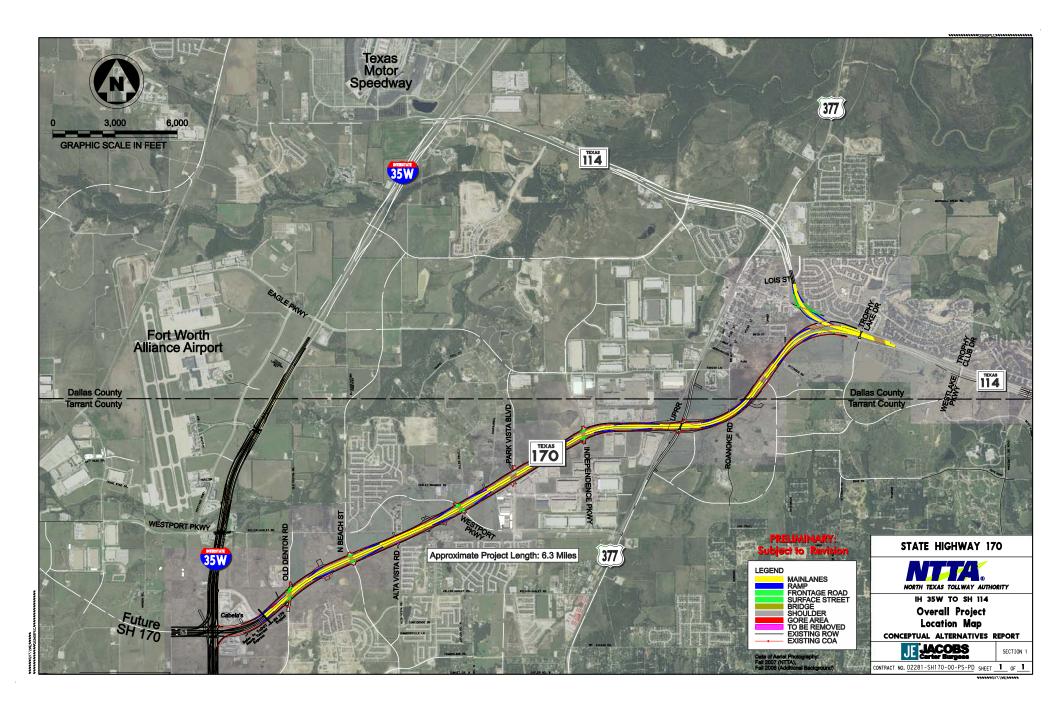
Number	Option Description	Mainlane Impacts	Ramp Impacts	Frontage Road Impacts	Access Control Impacts	Cost Impacts	Safety Impacts
1A	Move WB exit ramp to IH 35W (frontage road & Cabela Dr) to the east	Improves capacity due to further spacing of consecutive major exits from SH 170; also affected by Option 2	Improves weave to next ramp for IH 35W south, but reduces weave back to Beach St; depends on Option 2	Improves operations- moves ramp terminal further from Cabela Dr and IH 35W frontage roads	Improves access to Cabela Dr	Additional access rights	Removes access conflict, increases distance to signalized intersection
1B	Provide alternate access to driveway nearest to WB Westport ramp terminal	N/A	Ramp remains as planned	Removes access issue; requires construction of separate access drive	Removes access issue	Additional pavement required, possible additional ROW	Removes access conflict
1C	Provide alternate access method to driveway nearest to EB ramp to US 377	N/A	Ramp remains as planned	Removes access issue; requires construction of separate access drive	Removes access issue	Additional pavement required, possible additional ROW	Removes access conflict
1D	Provide alternate access method to driveway nearest to WB Independence ramp terminal	N/A	Ramp remains as planned	Removes access issue; requires construction of separate access drive	Removes access issue	Additional pavement required, possible additional ROW	Removes access conflict
2	Move entrance ramp from Beach St east approximately 1000'	Improves WB capacity approaching IH 35W; increases weave distance	Improves IH 35W ramp due to increase weave distance	Reduces distance from Beach St to ramp entry	N/A	Additional access rights	Mainlane weave distance increase should improve mainlane safety near major interchange
3	Depress Mainlanes, Build Permanent Bridge for Alta Vista	Gentler grade line, depressed section should reduce noise impacts to adjacent receivers	N/A	N/A	Access to local residential areas maintained as per existing	Additional cost of excavation, bridge for Alta Vista, and retaining walls.	Gentler mainlane grades improve visibility and capacity
4	Move WB Exit Ramp to Westport to the east	Slightly shortens mainlane weave	Ramp LOS may be affected by mainlane weave	N/A	Improves access to existing driveway	Additional access rights	Improves access to existing driveway
5	Reverse EB ramps between Independence & US 377	Places weave on mainlanes, would require auxiliary lane	Ramp LOS may be affected by mainlane weave	Weave removed from frontage road	Less access issues with existing driveways; requires exiting prior to Independence	Additional access rights	Frontage Road may be safer, Mainlanes weave may not be safer
6	Move WB Exit Ramp to Westport Pkwy to the east	Slightly shortens mainlane weave	Ramp LOS may be affected by mainlane weave	N/A	Removes access issue	Additional access rights	Improves access to existing driveway
7	Reverse WB ramps between US 377 & Independence	Places weave on mainlanes, would require auxiliary lane	Ramp LOS may be affected by mainlane weave	Weave removed from frontage road	Less access control issues with existing driveways, but requires exiting prior to US 377	Additional access rights	Frontage Road may be safer, Mainlanes weave may not be safer
8	Add U-turns at US 377	N/A	N/A	Improves Signal Operations	N/A	Additional 2 Bridges	Reduces Intersection Conflicts

Note: Favorable Ranking Construed as Positive Indicated by Gray Shading

APPENDIX



Regional Location Map





SH 170

Design Criteria From IH 35W to SH 114



Person Regulational of Transportations						
		Design Criteria				Design Criteria Location
	W 21212W 22		Frantona Banda	Cross Streets	_	
Item	Mainlanes	Direct Connectors/Ramps	Frontage Roads	Cross Streets	Reference	Page
item	Desirable Absolute Min	Desirable Absolute Min	Desirable Absolute Min			
Roadway Classification	Urban Freeway	Urban Freeway	Urban Arterial	See City thoroughfare Plans	RDM	Section 2-1
Design Speed	70 mph	50 mph 35 mph	45mph	30 mph min 45 mph desirable	RDM	Section 3-6 & Table 3-20
	•			•		
Horizontal Alignment						
Control Location	Roadway Centerline	Outside Lane Edge	Outside Face of Curb	Roadway Centerline		
Stopping Sight Distance	730'	425' 250	360'		RDM	Table 2-1
Minimum Centerline Radius	3405' 2050'	1055' 835'	950'		RDM	Table 2-3, Figure 2-2
Superelevation Rate	e(max) = 6%	e(max) = 6%	N/A		RDM	Table 2-6
Superelevation Runoff	0.40% relative gradient	0.50% relative gradient 0.62% relative gradient	N/A		RDM	Table 2-8
Vertical Alignment						
PGL/Axis of Location	Inside Lane Edge (SH 170) / Roadway CL (SH 114)	Outside Lane Edge	Outside FOC (Top of Pavement)			
Longitudinal Gradient	0% Min [1] 3.00% Max	0% Min [1] 5.00% Max	0.35% (curbed) Min 7% Max		RDM	Table 2-9 & Section 2-5, page 2-34
Longitudinal Gradient @Toll Plazas	0.50% 1.00% Max	0.50% 1.00% Max	N/A		NTTA Preference	
K Value for Crest Curves, min	247	84 29	61		RDM	Figure 2-9
K Value for Sag Curves, min	181	96 49	79		RDM	Figure 2-11
Grade change without a vertical curve	0.50 % max	0.50 % max 1.0% max	1.0 % max		RDM	Section 2-5
Vertical Clearance	5,550,75,1,500	0.00 // max			13000	COMOTE C
Over Roadways	16' - 6"	16' - 6"	16' - 6"		RDM	Table 3-1
	15. 5				-,	
Cross-Sectional Elements						
Widths of Travel Lanes	12'	One Lane 14', Two Lane 24' One Lane 14', 22' Curbed	12'	10! decimble: 11! min	RDM	Table 3-1 & Table 3-18
Number of mainlanes at ground level	12' 2 lanes with 1 auxiliary lane on each side	One Lane 14', Iwo Lane 24' One Lane 14', 22' Curbed N/A	12' N/A	12' desirable; 11' min. N/A	NTTA Preference	rable 3-1 & Table 3-18
	∠ ianes with i auxiliary lane on each side	N/A	N/A	N/A	NTTA Preterence	
Shoulder Widths Inside	12' [2]	2' on Roadway; 4' on Structure	N/A	N/A	RDM	Table 3-18
	12' [2] 10'	2 on Roadway; 4 on Structure	N/A N/A	N/A N/A	RDM RDM	Table 3-18
Outside Offset to face of curb	10° N/A	8' 6'	N/A 1'	N/A 2' desirable	RDM RDM	Table 3-18 Table 3-1
Cross Slope (Lane & Shoulder)	2.5% / 2.6%	2.5%			RDM	Section 2-6, Page 2-42
Medians Cross Slope (Lane & Shoulder)	2.5% / 2.0%	2.5%	2.6% [3]	2.0%	KUM	Section 2-6, Page 2-42
	depressed, divided	N/A	N/A	N/A	_ TxDot/NTTA Preference	
Type Width		N/A N/A	N/A N/A	N/A N/A	RDM	Section 3-5, Page 3-45 Section 3-6, Page 3-69
Traffic Safety Protection	50'	N/A N/A	N/A N/A	N/A N/A	TxDot/NTTA Preference	Section 3-5, Page 3-45 Section 3-6, Page 3-69
Monolithic Curbs	SSCB on one side - side to be determined	N/A	N/A	N/A	5 IXDOVINTTA Preference	
	N/A				TxDot/NTTA Preference	
Inside Outside	N/A N/A	none @ direct connectors : yes @ ramps [4] none @ direct connectors : yes @ ramps [4]	yes	yes	TxDot/NTTA Preference	
	N/A 30'	none @ direct connectors : yes @ ramps [4]	yes 1.5' min, 3' desirable	yes	RDM	Table 2-11
Clear Zone Width	30'	16"	1.5' min, 3' desirable	1.5' min, 3.0' desirable	KDM	Table 2-11
Side Slopes Within Clear Zone	10:1 desirable in Median, 6:1 max	6:1 max	4:1 max	6:1 max	RDM	0 1 00 0 0 10
		3:1 max			RDM RDM	Section 2-6, Page 2-43
Outside Clear Zone	3:1 max 1:10 max	3:1 max 1:10 max	3:1 max 1:10 max	4:1 max 1:10 max	RDM RDM	Section 2-6, Page 2-43
Through guard rail Sidewalk Width	N/A	1:10 max N/A	5' min, 6, desirable	5' min, 6, desirable	Ø RDM	Section 2-6, Page 2-43 Section 2-6, Page 2-46
	N/A	N/A	Match Existing	Match Existing	RDM	
Border Width Concrete Pvmt. Thickness	To be Determined	To be Determined	To be Determined	City Criteria	8 RDM	Table 3-1
Concrete Fyrit. Triickness	To be Determined	To be Determined	To be Determined	Oity Officia	σ	
Intersections						
Corner Radii						
Major Cross Streets	N/A	N/A	N/A	50' desirable	RDM	Section 7-7, Page 7-14
Minor Cross Streets	N/A	N/A	N/A	30' desirable	RDM	Section 7-7, Page 7-14
Driveways	N/A	N/A	N/A	20' desirable	RDM	Section 7-7, Page 7-14
Deisgn Vehicles						
Structural Desing	HL-93	HL-93	HL-93		LRFD	Section 2-2, Page 2-3
Horizontal Geometry	N/A	N/A	N/A		RDM	
Hydraulic Design Frequency						
Inles and Drainage Pipe	10 year	10 year	5 year		HDM	Section 5-3, Page 5-12
Inlets for Depressed Roadways	50 year	50 year	25 year		HDM	Section 5-3, Page 5-12 Section 5-3, Page 5-12
Culvert Design	50 year	50 year	10 year		HDM	Section 5-3, Page 5-12 Section 5-3, Page 5-12
	50 year	50 year	50 year (major river crossing) 25 year (small bridges)		HDM	Section 5-3, Page 5-12 Section 5-3, Page 5-12
Bridge Design Flood Check Frequency	100 year	100 year	100 year (small bridges)		HDM	Section 5-3, Page 5-12 Section 5-3, Page 5-12
1 lood offect riequelity	100 year	Too year	100 year		HUM	38000113-3, Fage 3-12
Hudrologie Method						
Hydrologic Method					11011	5. 5.0 5.0
Drainage Area < 200 ac	Rational Method	Rational Method	Rational Method		HDM	Figure 5-3, Page 5-21
Drainage Area > 200 ac	NRCS Unit Hydrograph, Regression Equations	NRCS Unit Hydrograph, Regression Equations	NRCS Unit Hydrograph, Regression Equations		HDM	Figure 5-3, Page 5-21
Culverts						
Headwater Control Location	Bottom of Paving Section	Bottom of Paving Section	Bottom of Paving Section	Bottom of Paving Section		
Outfall Velocity	2 fps 6 fps (w/o protection)	2 fps 6 fps (w/o protection)	2 fps 6 fps (w/o protection)	2 fps min 6 fps max (w/o protectoin)	HDM	Section 8-2, Page 8-12
Storm Drainage						
Max allowable Ponding Width	Shld + 1/2 of outside lane	Shoulder	Outer Lane	One Lane		Section 10-2, Page 10.9
Pipe Size	24"	24"	24"	24"	TxDot / NTTA / City	
Pipe Velocity	2.0 fps 12.0 fps max	2.0 fps 12.0 fps max	2.0 fps 12.0 fps max	2.0 fps min 12.0 fps max	HDM	Section 10-2, Page 10.10
Pipe Material	Reinforced Concrete Pipe	Reinforced Concrete Pipe	Reinforced Concrete Pipe	Reinforced Concrete Pipe	TxDot / NTTA	
			* * * * * * * * * * * * * * * * * * *			

Notes: RDM = TxDOT on Line Roadway Design Manual; HDM = TxDOT On Line Hydraulic Design Manual; LRFD = TxDOT On Line LRFD Bridge Design Manual.

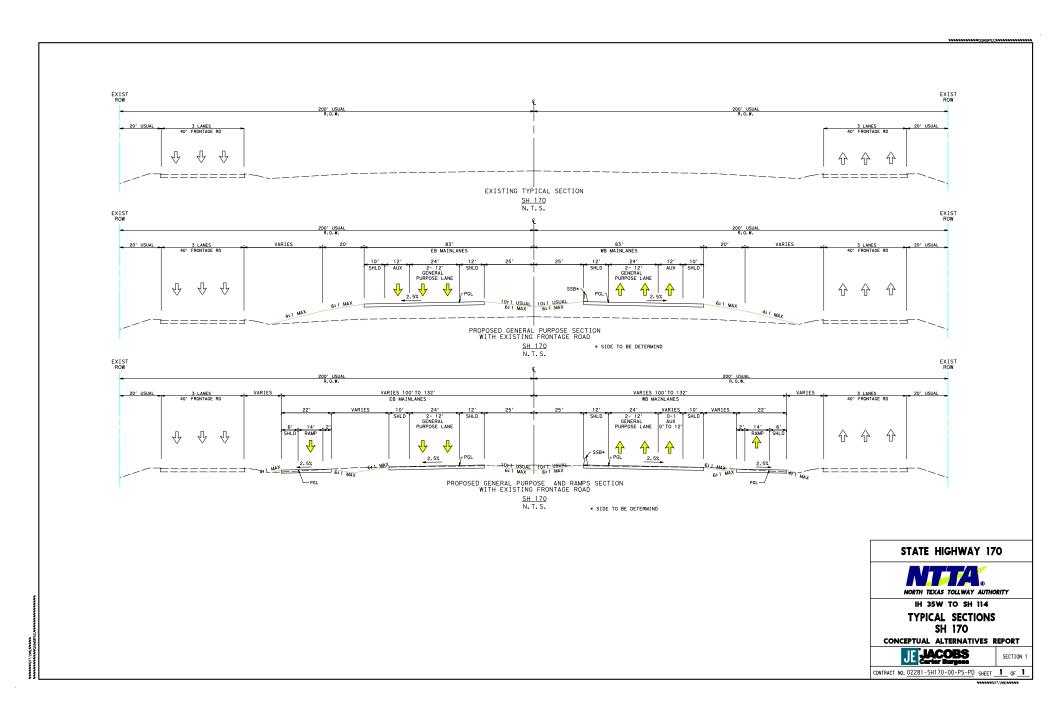
[1] If gradient less than 0.35% is used along non-curbed roadways, pavement cross-slope must provide adequate lateral drainage.

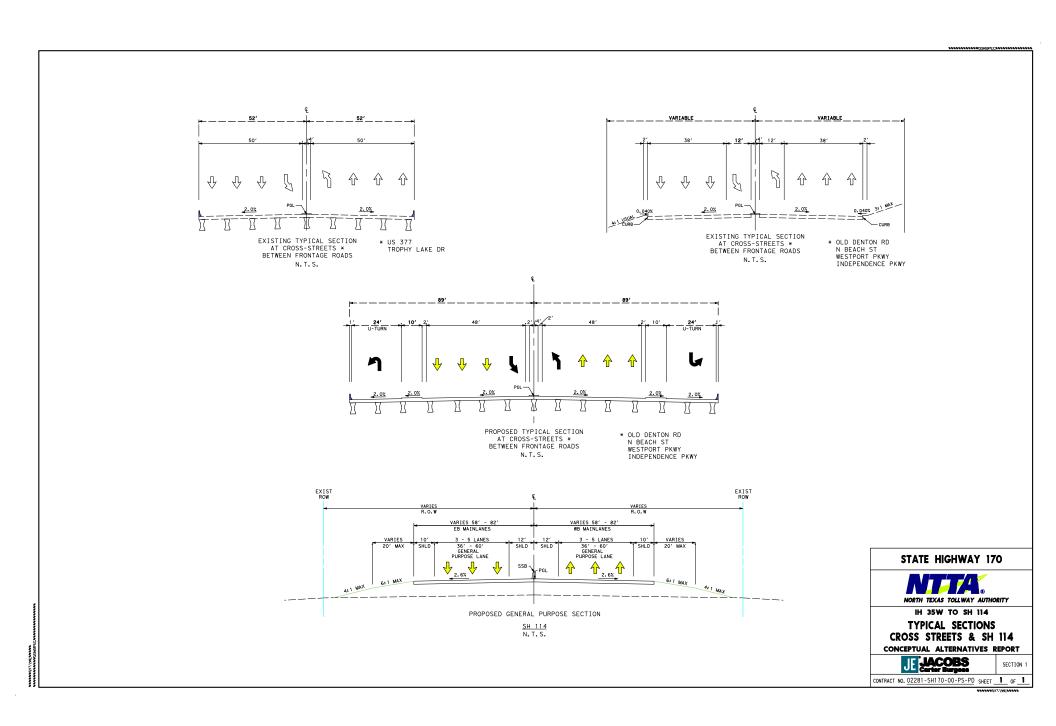
[2] To consider future expansion.

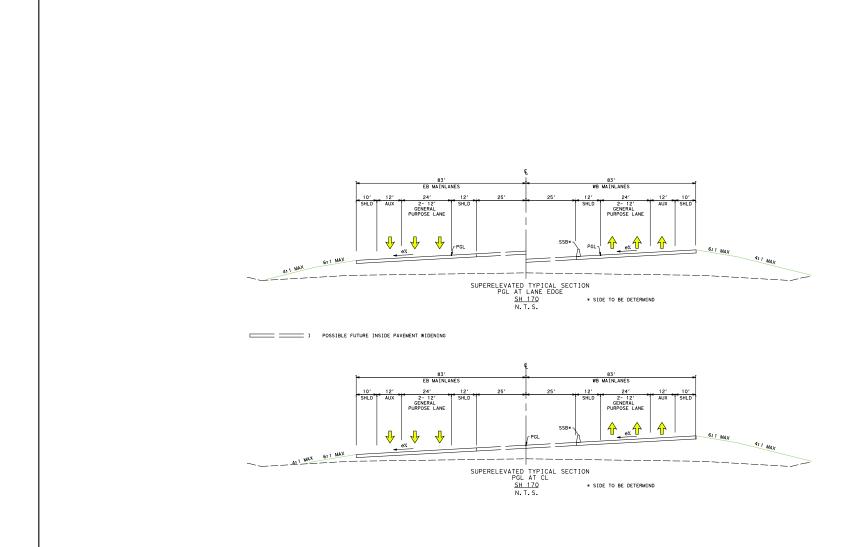
[3] Refer to plane SC333599-02-002, etc.

[4] At frontage roads

Revised: February, 2008







STATE HIGHWAY 170



IH 35W TO SH 114

TYPICAL SECTIONS ROTATION POINT COMPARISON CONCEPTUAL ALTERNATIVES REPORT

CONTRACT NO. 02281-SH170-00-PS-PD SHEET 1 OF 1

SECTION 1

